

Three-Dimensional Vibration of an Isotropic Plate Enclosed in a Rigid Body

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ABSTRACT

In this study, vibration of plates embedded in a rigid enclosure has been investigated analytically for the first time in the literature. It is assumed that the isotropic plate is always in contact with outer enclosure. Therefore, the normal displacement at a boundary surface is constrained but tangential displacement at a surface is allowed. The displacement field is assumed in trigonometric function form. This analytical solution is the only available exact solution of three-dimensional isotropic plate. Numerical results were presented for various geometrical parameters. It is believed that the present formulation and the results can be used as a benchmark for the numerical methods where the exact solution is not possible.

Keywords:

Vibration; Three-dimensional isotropic plate; Exact solution; Plate enclosed in a rigid; Trigonometric function form.

INTRODUCTION

Plates are used in the many engineering applications such as civil, mechanical and aerospace. In the service conditions, they are used as dynamic conditions. Therefore, it is an important point to understand the dynamic behaviour of the plate like structures.

Vibration of the isotropic plates were considered in many of the previous studies. They were summarized in the work of Leissa [1-3]. Different theories were developed in order to study vibration of plates. The classical plate theory is the first model developed by Kirchhoff [4]. In this theory, it is assumed that the normal lines before the deformation will be normal to the mid-surface of the plate after deformation. This theory is acceptable for the isotropic thin plates. In order to improve the accuracy the first and higher order shear deformation theories were proposed by some authors (see for example: Mindlin [5], Reissner [6] for the first order shear deformation theory and Reddy [7], Touratier [8], Soldatos [9] theories for the higher order models.). Noor and Burton [10] developed three-dimensional solutions for the free vibration and buckling analysis of multilayered angle-ply composite plates. Vibration of delaminated composite plates has been investigated using three-dimensional theory of linear elasticity in [11]. A finite element method has been developed for vibration analysis of delaminated composite plates and obtained results were compared with the experimental observations in

that paper. Liew et al. [12] studied the vibration analysis of thick rectangular plates using a continuum three-dimensional Ritz formulation. In another study [13], static and dynamic analyses of thick laminated plates have been examined using three-dimensional theory of linear elasticity and approximate Ritz method. Chen and Lü [14] investigated the three-dimensional free vibration analysis of cross-ply laminated plates. They used differential quadrature method in solution of the problem. The free vibration analysis of cross-ply laminated rectangular plates with clamped boundaries has been investigated in the framework of the three-dimensional elasticity in [15]. The results obtained from that paper were given for either isotropic or cross-ply laminated plates having different combinations of simply supported and clamped boundaries. Three-dimensional elasticity solutions have been presented for the free vibrations of rectangular plates based on the differential quadrature method in [16]. So and Leissa [17] studied the three-dimensional vibration analysis of thick circular and annular plates. The flexural thickness-shear, in-plane stretching, and torsional modes are included in the analyses in that paper. Liew and Yang [18] investigated the vibrational characteristics of annular plates based on the three-dimensional elasticity theory. Recently, Huang et al. [19] have developed a three-dimensional exact solution for the vibration analysis of functionally graded rectangular plates. The free vibrations of three-dimensional orthotropic rectangular plates have been

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investigated by Wang et al. [20]. An extended separation of variable method for the free vibration of orthotropic rectangular plates has been presented in that study. In addition to the studies mentioned above, there are some important papers related to mechanical analyses of one-dimensional, two-dimensional and three-dimensional structures [21-25].

It is well known that analytical closed form solutions are possible for all edge simply supported plates in two-dimensional plate theories. This solution method is known as Navier-type solution method in which trigonometric displacement functions are used in the displacement field in the solution. Levy type solution method is proposed for plates with at least two opposite simply supported edges. In this method, trigonometric functions are used for simply supported boundaries and equations of motion are converted to the ordinary differential equations. Then, they are solved using remaining boundary conditions. Other than these six boundary conditions, the remaining fifteen boundary conditions can not be solved analytically. Three-dimensional vibration of plates is a complicated problem and there is no analytical solution for many of the boundary conditions. Vibration of plates embedded in a rigid enclosure has been investigated analytically in the present study. These kind of dynamic problems can be encountered in machine elements or in some manufacturing processes like forging. This problem has not been investigated previously in the open literature. An isotropic plate is inserted in a rigid enclosure. Isotropic material properties were considered for the elastic plate and it is always in contact with outer enclosure. So, the normal displacement at a boundary surface is constrained but tangential displacement at a surface is allowed. The displacement fields are assumed in trigonometric function form since they satisfy the equations of motion and boundary conditions. Numerical results were presented for various geometrical parameters.

THEORY

The equations of motion of a three-dimensional plate for free vibration are defined as

$$\frac{\partial \sigma_x}{\partial x} + \frac{\partial \tau_{xy}}{\partial y} + \frac{\partial \tau_{xz}}{\partial z} = \rho \frac{\partial^2 u(x,t)}{\partial t^2} \tag{1}$$

$$\frac{\partial \tau_{xy}}{\partial x} + \frac{\partial \sigma_y}{\partial y} + \frac{\partial \tau_{yz}}{\partial z} = \rho \frac{\partial^2 v(x,t)}{\partial t^2} \tag{2}$$

$$\frac{\partial \tau_{xz}}{\partial x} + \frac{\partial \tau_{yz}}{\partial y} + \frac{\partial \sigma_z}{\partial z} = \rho \frac{\partial^2 w(x,t)}{\partial t^2} \tag{3}$$

where ρ is the mass density, t is the time, σ_x , σ_y and σ_z are normal stress in x,y and z -axes, τ_{xy} , τ_{yz} and τ_{xz} are shear stress in $x-y$, $y-z$ and $x-z$ planes, and u, v, w are the displacement components in x, y and z directions, res-

pectively. By using small strain assumption, normal and shear strain displacement relations are written as

$$\begin{aligned} \epsilon_x &= \frac{\partial u}{\partial x}, \epsilon_y = \frac{\partial v}{\partial y}, \epsilon_z = \frac{\partial w}{\partial z}, \\ \gamma_{xy} &= \frac{\partial u}{\partial y} + \frac{\partial v}{\partial x}, \gamma_{yz} = \frac{\partial v}{\partial z} + \frac{\partial w}{\partial y}, \gamma_{xz} = \frac{\partial u}{\partial z} + \frac{\partial w}{\partial x} \end{aligned} \tag{4}$$

Here, ϵ_x, ϵ_y and ϵ_z represent the normal strains and γ_{xy}, γ_{yz} and γ_{xz} represent the shear strains. For an isotropic plate, stress-strain relations can be written using Hooke's law as follows

$$\sigma_x = \lambda e + 2G\epsilon_x \tag{5}$$

$$\sigma_y = \lambda e + 2G\epsilon_y \tag{6}$$

$$\sigma_z = \lambda e + 2G\epsilon_z \tag{7}$$

$$\tau_{xy} = \frac{\gamma_{xy}}{G} \tag{8}$$

$$\tau_{yz} = \frac{\gamma_{yz}}{G} \tag{9}$$

$$\tau_{xz} = \frac{\gamma_{xz}}{G} \tag{10}$$

where

$$e = \epsilon_x + \epsilon_y + \epsilon_z \tag{11}$$

$$\lambda = \frac{\nu E}{(1+\nu)(1-2\nu)} \tag{12}$$

$$G = \frac{\nu E}{2(1+\nu)} \tag{13}$$

Here, λ denotes the Lamé constant, G denotes the shear modulus, E denotes the Young's modulus and ν denotes the Poisson's ratio. Inserting Eqs. (5), (6), (7) and (12) into Eqs. (1-3) leads to

$$(\lambda + G) \frac{\partial e}{\partial x} + G \nabla^2 u(x,t) = \rho \frac{\partial^2 u(x,t)}{\partial t^2} \tag{14}$$

$$(\lambda + G) \frac{\partial e}{\partial y} + G \nabla^2 v(y,t) = \rho \frac{\partial^2 v(y,t)}{\partial t^2} \tag{15}$$

$$(\lambda + G) \frac{\partial e}{\partial z} + G \nabla^2 w(z,t) = \rho \frac{\partial^2 w(z,t)}{\partial t^2} \tag{16}$$

where ∇^2 is the three-dimensional Laplacian operator which is defined as

$$\nabla^2 = \frac{\partial^2}{\partial x^2} + \frac{\partial^2}{\partial y^2} + \frac{\partial^2}{\partial z^2} \tag{17}$$

The boundary conditions which involved combination of displacements and stresses on any face can be used for the present problem. In this context, the exact vibration solution of Eqs. 14-17 can be assumed as

$$u(x,y,z,t) = A \sin(\alpha x) \cos(\beta y) \cos(\gamma z) \sin \omega t \tag{18a}$$

$$v(x, y, z, t) = B \cos(\alpha x) \sin(\beta y) \cos(\gamma z) \sin \omega t \quad (18b)$$

$$w(x, y, z, t) = C \cos(\alpha x) \cos(\beta y) \sin(\gamma z) \sin \omega t \quad (18c)$$

where

$$\alpha = \frac{m\pi}{a}, \beta = \frac{n\pi}{b}, \gamma = \frac{p\pi}{h}. \quad (19)$$

Here, ω is the circular frequency, m, n, p are integers which represent the half wave number, a, b, h are the length components of the isotropic three-dimensional plate about x, y and z -axis, respectively.

These boundary conditions can be considered as “an elastic block is enclosed by a much more rigid outer material or block which is well lubricated contact surface between two bodies” as stated by Leissa [1] (p.316) (Fig. 1).

It should be noted that an initial stress is required for continuous contact between elastic plate and outer enclosure. Although this boundary conditions are not common, it can be considered in a manufacturing process of a closed die forging. A more important point is the present results can be considered as a benchmark solution for the approximate solutions such as finite element method, finite difference method, differential quadrature method, etc.

Then, substituting Eqs. (18) and (19) into the equations of motion (14-16) gives a sixth-order characteristic equation of the present vibration problem of three-dimensional isotropic plate enclosed in a rigid body with the following equation:

$$\begin{bmatrix} K_{11} & K_{12} & K_{13} \\ K_{21} & K_{22} & K_{23} \\ K_{31} & K_{32} & K_{33} \end{bmatrix} \begin{bmatrix} A \\ B \\ C \end{bmatrix} = \begin{bmatrix} 0 \\ 0 \\ 0 \end{bmatrix} \quad (20)$$

For a non-trivial solution, the determinant of the coefficient matrix in the previous equation (20) must be zero. Thus, it can be obtained the sixth-order frequency determinant. The positive roots of the sixth-order frequency determinant obtained from Eq. (20) give three positive frequencies of isotropic three-dimensional plate. These three positive vibration frequencies represent the first, second and third spectrum frequencies for each half wave number. It should be noted that the first and se-

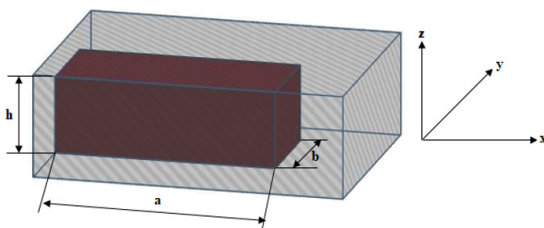


Figure 1. The structure of the model.

cond spectrum frequencies give same values for the present problem. The elements of the coefficient matrix of Eq. (20) can be obtained in dimensionless form as follows:

$$\begin{aligned} K_{11} &= \bar{\alpha}^2 - 2\frac{G}{\lambda}\bar{\alpha}^2 - \frac{G}{\lambda}k_1^2\bar{\beta}^2 - \frac{G}{\lambda}k_2^2\bar{\gamma}^2 + \Omega^2\bar{D}, \\ K_{12} &= -k_1\bar{\alpha}\bar{\beta} - \frac{G}{\lambda}k_1\bar{\alpha}\bar{\beta}, \\ K_{13} &= -k_2\bar{\alpha}\bar{\gamma} - \frac{G}{\lambda}k_2\bar{\alpha}\bar{\gamma}, \\ K_{21} &= -k_1\bar{\alpha}\bar{\beta} - \frac{G}{\lambda}k_1\bar{\alpha}\bar{\beta}, \\ K_{22} &= -\bar{\alpha}^2\frac{G}{\lambda} - k_1^2\bar{\beta}^2 - \frac{2G}{\lambda}k_1^2\bar{\beta}^2 - \frac{G}{\lambda}k_2^2\bar{\gamma}^2 + \Omega^2\bar{D}, \\ K_{23} &= -k_1k_2\bar{\beta}\bar{\gamma} - \frac{G}{\lambda}k_1k_2\bar{\beta}\bar{\gamma}, \\ K_{31} &= -k_2\bar{\alpha}\bar{\gamma} - \frac{G}{\lambda}k_2\bar{\alpha}\bar{\gamma}, \\ K_{32} &= -k_1k_2\bar{\beta}\bar{\gamma} - \frac{G}{\lambda}k_1k_2\bar{\beta}\bar{\gamma}, \\ K_{33} &= -\bar{\alpha}^2\frac{G}{\lambda} - k_2^2\bar{\gamma}^2 - \frac{G}{\lambda}k_1^2\bar{\beta}^2 - \frac{2G}{\lambda}k_2^2\bar{\gamma}^2 + \Omega^2\bar{D}. \end{aligned} \quad (21)$$

where dimensionless terms are

$$\bar{\alpha} = \frac{\alpha}{a}, \bar{\beta} = \frac{\beta}{b}, \bar{\gamma} = \frac{\gamma}{y}, k_1 = \frac{a}{b}, k_2 = \frac{a}{h}, \bar{D} = \frac{D}{\lambda ha^2}. \quad (22)$$

and the dimensionless frequency parameter (Ω^2) of three-dimensional isotropic plate is defined as

$$\Omega = \omega a^2 \sqrt{\frac{\rho h}{D}}, D = \frac{Eh^3}{12(1-\nu^2)}. \quad (23)$$

The roots of the characteristic equations obtained from Eq. (20) are

$$\Omega_{mnp,1} = \Omega_{mnp,2} = \frac{\sqrt{DG\lambda(\bar{\beta}^2 k_1^2 + \bar{\alpha}^2 + \bar{\gamma}^2 k_2^2)}}{\bar{D}\lambda} \quad (24a)$$

$$\Omega_{mnp,3} = \frac{\sqrt{\bar{D}\lambda(2G + \lambda)(\bar{\beta}^2 k_1^2 + \bar{\alpha}^2 + \bar{\gamma}^2 k_2^2)}}{\bar{D}\lambda} \quad (24b)$$

Here, $\Omega_{mnp,1}$, $\Omega_{mnp,2}$ and $\Omega_{mnp,3}$ denote the first, second and third set frequency roots of isotropic three-dimensional plate for each half wave number.

RESULTS AND DISCUSSION

The numerical results of the formulation presented in Section 2 are given for various geometrical parameters. Three sets of frequencies are obtained for a given set of wave numbers (m, n, p). The frequencies with only one wave number in one of the coordinate axes can be obtained by selecting the corresponding half wave integer (m, n, p) as a nonzero integer and zero for other integers in Table 1a. They are Ω_{100} , Ω_{010} and Ω_{001} frequencies. These cases are pure axial deformations along the correspon-

Table 1a. The dimensionless vibration frequencies (Ω) for different mode numbers ($\Omega_{100}, \Omega_{010}, \Omega_{001}$).

		Ω_{100}	
a/h	a/b	$\Omega_{100,2}(\Omega_{100,2})$	$\Omega_{100,3}$
5	1	32.1917	60.2252
	2	32.1917	60.2252
	3	32.1917	60.2252
	5	32.1917	60.2252
20	1	128.7669	240.9009
	2	128.7669	240.9009
	3	128.7669	240.9009
	5	128.7669	240.9009
100	1	643.8349	1204.5048
	2	643.8349	1204.5048
	3	643.8349	1204.5048
	5	643.8349	1204.5048
		Ω_{010}	
a/h	a/b	$\Omega_{010,2}(\Omega_{010,2})$	$\Omega_{010,3}$
5	1	32.1917	60.2252
	2	64.3834	120.4504
	3	96.5752	180.6757
	5	160.9587	301.1262
20	1	128.7669	240.9009
	2	257.5339	481.8019
	3	386.3009	722.7028
	5	643.8349	1204.5048
100	1	643.8349	1204.5048
	2	1287.6698	2409.0096
	3	1931.5047	3613.5144
	5	3219.1745	6022.5240
		Ω_{001}	
a/h	a/b	$\Omega_{001,2}(\Omega_{001,2})$	$\Omega_{001,3}$
5	1	160.9587	301.1262
	2	160.9587	301.1262
	3	160.9587	301.1262
	5	160.9587	301.1262
20	1	2575.3396	4818.0192
	2	2575.3396	4818.0192
	3	2575.3396	4818.0192
	5	2575.3396	4818.0192
100	1	64383.4904	120450.4814
	2	64383.4904	120450.4814
	3	64383.4904	120450.4814
	5	64383.4904	120450.4814

Table 1b. The dimensionless vibration frequencies (Ω) for different mode numbers ($\Omega_{110}, \Omega_{101}, \Omega_{011}$).

		Ω_{110}	
a/h	a/b	$\Omega_{110,2}(\Omega_{110,2})$	$\Omega_{110,3}$
5	1	45.5260	85.1713
	2	71.9829	134.6677
	3	101.7992	190.4489
	5	164.1463	307.0896
20	1	182.1040	340.6854
	2	287.9317	538.6709
	3	407.1969	761.7957
	5	656.5853	1228.3587
100	1	910.5200	1703.4270
	2	1439.6586	2693.3546
	3	2035.9847	3808.9786
	5	3282.9267	6141.7935
		Ω_{101}	
a/h	a/b	$\Omega_{101,2}(\Omega_{101,2})$	$\Omega_{101,3}$
5	1	164.1463	307.0896
	2	164.1463	307.0896
	3	164.1463	307.0896
	5	164.1463	307.0896
20	1	2578.5567	4824.0380
	2	2578.5567	4824.0380
	3	2578.5567	4824.0380
	5	2578.5567	4824.0380
100	1	64386.7095	120456.5037
	2	64386.7095	120456.5037
	3	64386.7095	120456.5037
	5	64386.7095	120456.5037
		Ω_{011}	
a/h	a/b	$\Omega_{011,2}(\Omega_{011,2})$	$\Omega_{011,3}$
5	1	164.1463	307.0896
	2	173.3578	324.3228
	3	187.7085	351.1704
	5	227.6300	425.8567
20	1	2578.5567	4824.0380
	2	2588.1842	4842.0494
	3	2604.1510	4871.9204
	5	2654.5993	4966.3005
100	1	64386.7095	120456.5037
	2	64396.3659	120474.5690
	3	64412.4565	120504.6719
	5	64463.9196	120600.9505

Table 1c. The dimensionless vibration frequencies (Ω) for different mode numbers ($\Omega_{221}, \Omega_{211}, \Omega_{121}$).

		Ω_{221}	
a/h	a/b	$\Omega_{221,1}(\Omega_{221,2})$	$\Omega_{221,3}$
5	1	184.9274	345.9676
	2	215.9487	404.0031
	3	259.5381	485.5514
	5	365.6279	684.0272
20	1	2600.9655	4865.9609
	2	2638.9378	4937.0006
	3	2701.0389	5053.1812
	5	2890.8115	5408.2132
100	1	64409.2387	120498.6519
	2	64447.8418	120570.8717
	3	64512.1289	120691.1419
	5	64717.4186	121075.2038
		Ω_{211}	
a/h	a/b	$\Omega_{211,1}(\Omega_{211,2})$	$\Omega_{211,3}$
5	1	176.3214	329.8672
	2	184.9274	345.9676
	3	198.4432	371.2533
	5	236.5600	442.5633
20	1	2591.3855	4848.0383
	2	2600.9655	4865.9609
	3	2616.8542	4895.6860
	5	2667.0622	4989.6166
100	1	64399.5843	120480.5902
	2	64409.2387	120498.6519
	3	64425.3261	120528.7487
	5	64476.7789	120625.0081
		Ω_{121}	
a/h	a/b	$\Omega_{121,1}(\Omega_{121,2})$	$\Omega_{121,3}$
5	1	176.3214	329.8672
	2	208.6263	390.3041
	3	253.4780	474.2140
	5	361.3514	676.0266
20	1	2591.3855	4848.0383
	2	2629.4961	4919.3368
	3	2691.8151	5035.9250
	5	2882.1951	5392.0933
100	1	64399.5843	120480.5902
	2	64438.1932	120552.8208
	3	64502.4899	120673.1090
	5	64707.8102	121057.2281

Table 1d. The dimensionless vibration frequencies (Ω) for different mode numbers ($\Omega_{331}, \Omega_{321}, \Omega_{231}$).

		Ω_{331}	
a/h	a/b	$\Omega_{331,1}(\Omega_{331,2})$	$\Omega_{331,3}$
5	1	211.0953	394.9233
	2	269.3354	503.8805
	3	345.2180	645.8437
	5	518.0771	969.2335
20	1	2632.6471	4925.2318
	2	2716.3424	5081.8113
	3	2850.3786	5332.5701
	5	3242.2697	6065.7312
100	1	64441.4095	120558.8380
	2	64528.1907	120721.1907
	3	64672.5672	120991.2944
	5	65132.4214	121851.6028
		Ω_{321}	
a/h	a/b	$\Omega_{321,1}(\Omega_{321,2})$	$\Omega_{321,3}$
5	1	198.4432	371.2533
	2	227.6300	425.8567
	3	269.3354	503.8805
	5	372.6463	697.1575
20	1	2616.8542	4895.6860
	2	2654.5993	4966.3005
	3	2716.3424	5081.8113
	5	2905.1155	5434.9734
100	1	64425.3261	120528.7487
	2	64463.9196	120600.9505
	3	64528.1907	120721.1907
	5	64733.4295	121105.1573
		Ω_{231}	
a/h	a/b	$\Omega_{231,1}(\Omega_{231,2})$	$\Omega_{231,3}$
5	1	198.4432	371.2533
	2	259.5381	485.5514
	3	337.6298	631.6476
	5	513.0519	959.8323
20	1	2616.8542	4895.6860
	2	2701.0389	5053.1812
	3	2835.7985	5305.2933
	5	3229.4594	6041.7654
100	1	64425.3261	120528.7487
	2	64512.1289	120691.1419
	3	64656.5413	120961.3127
	5	65116.5086	121821.8328

ding directions. The frequencies are given in Table 1a for various a/h and a/b ratios for the first three frequencies. The frequencies for Ω_{100} and Ω_{001} are independent from a/h and a/b ratios.

Similar to one-dimensional waves, plane waves can be obtained by setting one of the half wave integer to zero. Therefore, the integers are Ω_{110} , Ω_{101} , Ω_{011} (Table 1b). For example Ω_{110} is a wave in the xy plane with zero flexure ($w=0$). Plane wave frequencies are higher than one-dimensional frequencies. This fact can be seen from Eq. (24).

Table 2a. The first three dimensionless vibration frequencies (Ω) for $a/b=1$.

First mode		
a/h	$\Omega_{111,1}(\Omega_{111,2})$	$\Omega_{111,3}$
1	11.1515	20.8626
2	31.5413	59.0084
3	64.0607	119.8467
5	167.2732	312.9395
10	650.2413	1216.4902
20	2581.7699	4830.0492
30	5800.9489	10852.5816
50	16102.3096	30124.6629
70	31554.3498	59032.7797
100	64389.9285	120462.5258
Second mode		
a/h	$\Omega_{222,1}(\Omega_{222,2})$	$\Omega_{222,3}$
1	22.3030	41.7252
2	63.0826	118.0168
3	128.1215	239.6934
5	334.5464	625.8790
10	1300.4827	2432.9804
20	5163.5398	9660.0985
30	11601.8978	21705.1633
50	32204.6193	60249.3259
70	63108.6997	118065.5594
100	128779.8570	240925.0516
Third mode		
a/h	$\Omega_{333,1}(\Omega_{333,2})$	$\Omega_{333,3}$
1	33.4546	62.5879
2	94.6240	177.0253
3	192.1822	359.5401
5	501.8196	938.8185
10	1950.7241	3649.4706
20	7745.3098	14490.1478
30	17402.8467	32557.7450
50	48306.9290	90373.9889
70	94663.0496	177098.3391
100	193169.7855	361387.5775

Table 2b. The first three dimensionless vibration frequencies (Ω) for $a/b=2$.

First mode		
a/h	$\Omega_{111,1}(\Omega_{111,2})$	$\Omega_{111,3}$
1	15.7706	29.5042
2	38.6301	72.2702
3	72.2702	135.2053
5	176.3214	329.8672
10	659.7344	1234.2501
20	2591.3855	4848.0383
30	5810.5877	10870.6142
50	16111.9604	30142.7179
70	31564.0021	59050.8408
100	64399.5843	120480.5902
Second mode		
a/h	$\Omega_{222,1}(\Omega_{222,2})$	$\Omega_{222,3}$
1	31.5413	59.0084
2	77.2601	144.5405
3	144.5405	270.4106
5	352.6429	659.7344
10	1319.4689	2468.5003
20	5182.7710	9696.0767
30	11621.1754	21741.2284
50	32223.9209	60285.4358
70	63128.0042	118101.6816
100	128799.1687	240961.1805
Third mode		
a/h	$\Omega_{333,1}(\Omega_{333,2})$	$\Omega_{333,3}$
1	47.3120	88.5126
2	115.8902	216.8108
3	216.8108	405.6159
5	528.9643	989.6016
10	1979.2033	3702.7504
20	7774.1565	14544.1150
30	17431.7631	32611.8427
50	48335.8813	90428.1537
70	94692.0063	177152.5224
100	193198.7530	361441.7707

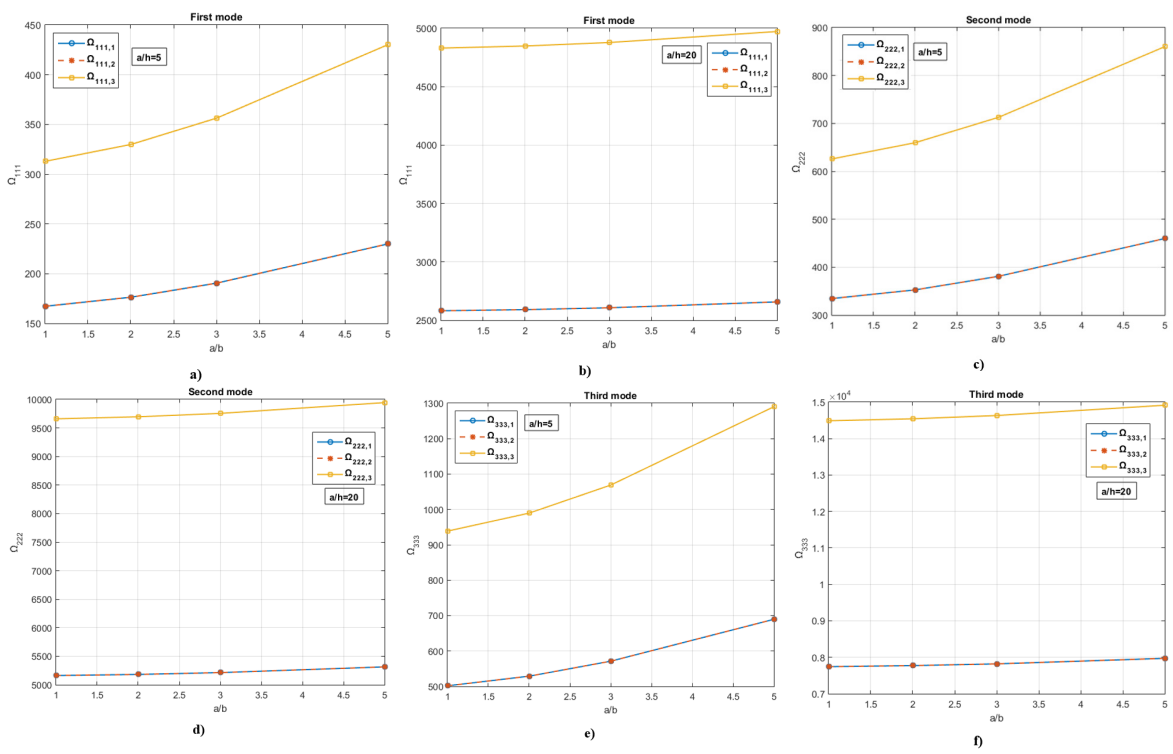


Figure 2. Variation of the first three dimensionless frequencies with a/b .

More general vibration case can be obtained by setting all of the wave number integers to a nonzero value. The corresponding results for various a/h and a/b ratios for the first three frequencies. The first three frequency parameters of plates are given in Tables 1 and 2 for various modes and a/h and a/b ratios. It is seen that first two sets of frequencies are identical for the all cases considered. This can be seen from Eq. (24). It is interesting to note that third set frequency is 1.87 times of the first set frequency approximately. The frequency parameters are increasing with a/h ratio.

The variation of first three frequencies parameters with a/b ratios were given in Fig. 2. The frequency parameters of thicker plates are more sensitive to a/b ratio when compared to thinner plates. It should be also noted that a/b ratio is also more effective for the higher modes of vibration. Plate outer surfaces are always in contact with the outer body. First two frequencies are in-plane dominated frequencies whereas the third frequency is flexural dominated frequency.

Some mode shapes of vibrating plates are given in Fig. 3. These mode shapes are given for $a/h=5$. For the first mode, there is a nodal line at $y=b/2$. The highest displacement is observed at $x=a/2$. The v -mode shape is obtained by rotating a 90° in the clockwise direction. The first mode shape of w is in a shape of saddle with the symmetry axis located at $y=-x+1$.

CONCLUSION

The analytical solution of isotropic three-dimensional plate in a rigid enclosure has been presented in the present study. Dimensionless frequencies of three-dimensional plate have been obtained for various geometrical values. Three sets of frequencies are obtained for each set of wave numbers and the first and second set frequencies become identical. It is obtained that one-dimensional frequencies Ω_{100} and Ω_{001} are independent from a/h and a/b ratios. Plane wave frequencies give higher values than one-dimensional frequencies. The frequencies of thicker plates are more sensitive to a/b ratio when compared to thinner plates. Two groups of frequencies have been observed as in-plane dominated (the first two set frequencies) and flexural dominated (third set frequency).

CONFLICT OF INTEREST

Authors approve that to the best of their knowledge, there is not any conflict of interest or common interest with an institution/organization or a person that may affect the review process of the paper

AUTHOR CONTRIBUTION

Ufuk Gul: Writing - original draft, Visualization, Investigation. Metin Aydogdu: Supervision, Conceptualization, Writing - review & editing.

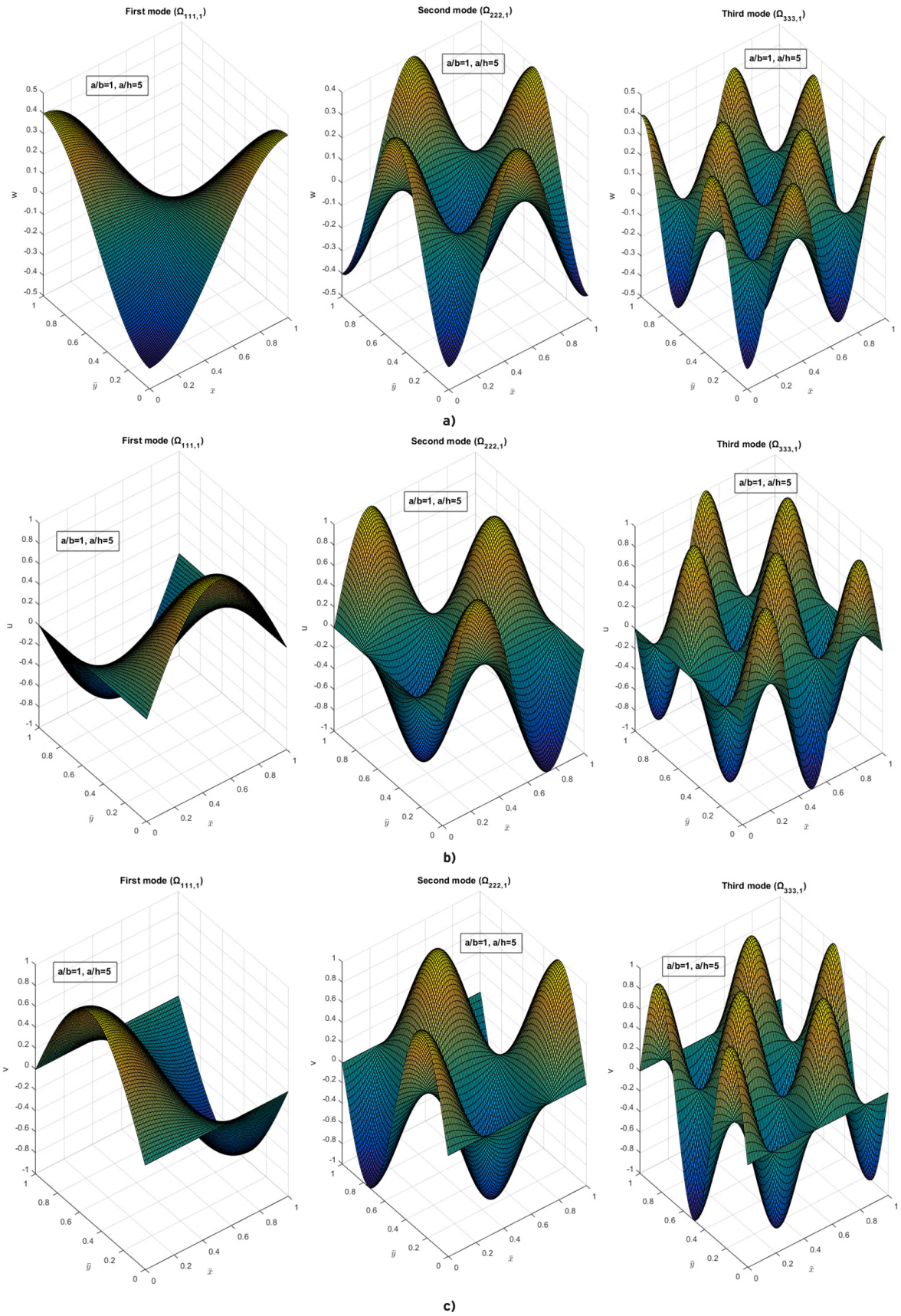


Figure 3. Mode shapes for $a/b=1$ and $a/h=5$.

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